

SWELLING OF BENTONITE-SAND MIXTURES AFTER LONG-TERM DISSOLUTION IN ALKALINE SOLUTION

GUOSHENG X IANG^{1,2} * AND WEIMIN YE^2

¹ Anhui University of Technology, Ma'anshan 243002, China
² Tongii University, Shanghai 200092, China Tongji University, Shanghai 200092, China

Abstract—Alkaline solutions have significant effects on the mineral composition and on the microstructure of bentonite; in relevant geoenvironmental engineering applications, therefore, the properties of bentonite buffer materials must be taken into consideration in the presence of alkaline solutions. The objective of the present study was to determine the effect of alkaline conditions on the swelling of bentonite mixed with sand. Bentonite-sand mixtures were soaked in a NaOH solution and allowed to react over prescribed periods of 6, 12, and 24 months. Swelling deformation tests were conducted on the alkali-treated bentonite-sand mixtures; the swelling of the mixtures decreased significantly with increased reaction time. The fractal e_c -σ relationship (e_c is void ratio of bentonite, σ is vertical stress) was employed to express the swelling characteristics of the alkali-treated mixtures, wherein the swelling coefficient decreased as the bentonite content was reduced. Dissolution traces over the clay surfaces degraded the microstructural phase, thereby slightly increasing the fractal dimension. At higher dosages of bentonite, the swelling of bentonite-sand mixtures always followed a similar e_c - σ relationship as that found for bentonite alone. On the contrary, in the mixtures with a small bentonite content that surpassed the designated threshold pressure, the void ratio of clay in the mixtures deviated from the e_c - σ curve due to the appearance of the sand skeleton. The bentonite content for a particular bentonite-sand mixture at which deviation from the e_c - σ curve began was \sim 50%. This deviation was almost negligible at 50% initial bentonite content in the bentonite-sand mixtures; after treatment with NaOH solution, however, a pronounced deviation in the e_c - σ curve was observed which was caused mainly by the decrease in the bentonite percentage. Finally, the vertical pressure threshold was also estimated using the e_c - σ relation for bentonite-sand mixtures with small bentonite contents over a range of various alkaline solution reaction times.

Keywords—Alkaline solution · Bentonite-sand mixture · Fractal e_c -σ relation · Reaction time · Swelling

INTRODUCTION

Owing to their low hydraulic conductivity, high swelling capacity, and great adsorption capacity, bentonite and sandbentonite mixtures have been used widely in solid waste landfills (Ashmawy et al. [2002\)](#page-6-0), municipal and industrial waste disposal sites (Hoeks et al. [1987](#page-7-0); Du et al. [2015](#page-6-0)), and deep geological disposal repositories of highly radioactive nuclear waste materials (Villar, [2007](#page-7-0); Hayakawa et al. [2016\)](#page-7-0). In these applications, alkaline leachates are relatively common and are discharged generally from production processes such as papermaking, dyeing, tanning, and metal processing. In addition, highly alkaline solutions can originate from the degradation of concrete occurring in landfills or nuclear repositories (Nakayama et al. [2004;](#page-7-0) Fernández et al. [2006\)](#page-7-0). The alkaline leachates tend to seep into bentonite buffers, thus interacting with bentonite which leads to significant attenuation in the overall characteristics of engineered buffer materials. An investigation into the attenuating impact of alkaline solutions on bentonite-based buffer materials is, therefore, important.

Several researchers (Ramírez et al. [2002](#page-7-0); Cuisinier et al. [2008;](#page-6-0) Ye et al. [2016](#page-7-0)) have noted that alkaline porewater has a significant effect on the properties of bentonites as it can dissolve the smectite in bentonites (Charpentiera et al. [2006](#page-6-0); Cuevas et al. [2006](#page-6-0); Gates and Bouazza, [2010](#page-7-0)) thereby forming

DOI: 10.1007/s42860-020-00090-w

secondary non-swelling minerals (Heikola et al. [2013\)](#page-7-0). Zeolites such as analcime and phillipsite were detected in the reaction of smectites with NaOH solutions at elevated temperatures (Nakayama et al. [2004](#page-7-0)). Furthermore, the smectites present in bentonites could transform to illite or illite-smectite layers in a potassium alkaline medium (Cuadros and Linares, [1996](#page-6-0); Gaucher and Blanc, [2006\)](#page-7-0). The presence of calcium silicate hydrate (C-S-H) gels was evidenced from the reactions of smectites in alkaline solutions containing portlandite $Ca(OH)$ ₂ (Ramírez et al. [2002](#page-7-0); Sánchez et al. [2006](#page-7-0)). As a result, the swelling characteristics of bentonite were largely vitiated by incorporation of the alkaline solution (Karnland et al. [2007\)](#page-7-0). Savage et al. ([2007](#page-7-0)) claimed that cement solutions enhanced the rate of dissolution of smectite while inhibiting the swelling of bentonite. Herbert et al. ([2008](#page-7-0)) studied the swelling behavior of the MX-80 bentonite (a natural sodic bentonite from Wyoming, USA) infiltrated with a variety of solutions and observed changes in the swelling patterns, changes that were caused mainly by variations in the amount of smectite-rich montmorillonite or kaolinite. Smectite in bentonite can also be dissolved continuously over time by alkaline solutions, which causes the degradation of the buffering properties of the bentonite (Sánchez et al. [2006](#page-7-0); Yamaguchi et al. [2007](#page-7-0)). For instance, Chen et al. [\(2019](#page-6-0)) argued that the increase in contact time with synthetic Portland cement solutions would decrease the swelling pressure of GMZ bentonite, which is a natural bentonite extracted from GaoMiaoZi county in the Inner Mongolia province, China.

Unlike bentonite alone, the swelling states of bentonitesand mixtures may also be affected by the sand content

^{*} E-mail address of corresponding author: xianggsh2011@163. com

(Mollins et al. [1996\)](#page-7-0). Bentonite-sand mixtures with large bentonite contents (α) attained the same void ratio, e_c , as that of pure bentonite (Sun et al. [2009](#page-7-0)), unlike the mixtures with small bentonite contents (Xu et al. [2003;](#page-7-0) Xiang et al. [2019a](#page-7-0)). At greater vertical pressure, the sand skeleton, which refers to an interconnected network in which the sand grains are touching each other, would form in the case of low- α mixtures while bearing part of the vertical stress, thus leading to a greater clay– void ratio. Due to smectite dissolution in the alkaline solutions, the smectite content in bentonites and the bentonite content in mixtures tended to decrease over the reaction time. The differences in respective void ratios exhibited greater tendencies to occur between the bentonite-sand mixtures and the pure bentonite. The swelling states of bentonite-sand mixtures generated due to reaction with alkaline solutions are poorly understood.

Numerous modeling studies of the swelling of bentoniteaggregate mixtures have been undertaken. Some fitting relationships were proposed based on the swelling-test results in order to describe the expansion mechanism of pure bentonite or the bentonite-sand mixtures as a function of either the final dry density (Wang et al. [2012](#page-7-0)) or the void ratio of smectites (Komine and Ogata, [1996\)](#page-7-0). Such expressions were limited to particular cases and, hence, have complexities when used for predicting swelling characteristics of other types of bentonites. By contrast, some eminent phenomenological models can be found in the literature, such as the modified Cam-Clay model (Roscoe and Burland [1968](#page-7-0)) and the Barcelona Model (Alonso et al. [1999\)](#page-6-0) that employ physico-chemical parameters to predict the macroscopic behavior of expansive clay soils. The aforementioned models are rather complex as they need a variety of parameters. Until now, diffuse double-layer (DDL) models have been used widely to describe the volumetric behavior of bentonite-based materials (Bolt [1956](#page-6-0); Komine and Ogata [1996](#page-7-0); Tripathy et al. [2004](#page-7-0)). The DDL models were found to be more efficient for Na-dominant bentonites in dilute solutions, however, in contrast to calcic bentonites of relatively high density (Schanz and Tripathy [2009\)](#page-7-0) or bentonites soaked in concentrated solutions (Yong [1999](#page-7-0)). Moreover, the DDL models do not explain effectively the swelling of bentonite-sand mixtures with large sand contents after the formation of a sand skeleton. According to Xu et al. ([2014](#page-7-0)), a fractal relationship exists between the void ratio of clay, e_c , and the vertical stress, σ, in distilled water, and this relation has been validated by the experimental data from numerous swell-ing clays (Xu et al. [2014;](#page-7-0) Sun et al. [2015](#page-7-0)). The e_c - σ relationship offers a simple method to predict the swelling of bentonite; the application of this relationship under the condition of alkaline dissolution is a question which still remains, however.

The objectives of the present study were, therefore, to investigate the effects of long-term alkaline dissolution on the swelling properties of bentonite-sand mixtures and to propose a method for estimating the variation in swelling, which will provide a theoretical direction for the performance evaluation of bentonite-based buffer materials under alkaline conditions.

MATERIALS AND METHODS

Materials

The bentonite and sand used in the present study were obtained from Henan Province, China. Their physical properties are listed in Table 1. The cation exchange capacity (CEC) of the bentonite was 0.517 meq/g and the exchange cations in meq/g were 0.344 Na^+ , 0.105 Ca^{2+} , 0.053 Mg^{2+} , and 0.015 K^+ . The Na⁺ was dominant in the bentonite tested. The swelling of bentonite depends mainly on the smectite content and on the exchangeable cations. The alkaline leachates may be of complex compositions that could lead to multiple chemical reactions with the bentonite. In this study, NaOH solution was used because cation replaceability occurs typically in the order Na+ $\langle K^+ \times Mg^{2+} \rangle \le Ca^{2+} \langle NH_4^+$. The exchange of other cations by Na+ was relatively difficult, so for the purposes of the present study the type of cation exchange was ignored while the effects of OH– were considered.

The bentonite powder and sand were oven dried at 105°C for 24 h, and were combined by weight at prescribed bentonite-sand ratios of 100:0, 70:30; 50:50, and 45:55. Then, bentonite and sand were inserted into a stainless steel drum and stirred for 30 min to achieve a uniform mix. After that, 1.0 kg of oven-dried bentonite or its sand mixture was stirred and mixed with 1.0 L of 1.0 mol/L NaOH solution, then sealed in a stainless steel reaction kettle with a Teflon inner sleeve for 6, 12, and 24 months. At the end of each reaction period, the solid and the liquid phases of bentonite-sand mixtures were separated using a centrifuge (TG 5427 R, Eppendorf, Merck, Darmstadt, Germany) operated at $3400 \times g$ for 30 min. The pH of the residual solution in the pure bentonite was measured after separation. The pH recorded was 13.80, 13.61, and 13.38 for pure bentonite dissolved by NaOH solution after 6, 12, and 24 months, respectively, whereas the residual concentration of OH– was 0.63, 0.41, and 0.25 mol/L, respectively.

Swelling Deformation Tests

After dissolution in the alkaline solution, the samples were washed carefully three times with water and then compacted inside a stainless steel ring (50 mm in diameter and 10 mm tall) at a loading rate of 0.2 kN/min. The applied load and displacements of the piston were recorded. When the target dry density of 1.70 γ /cm³ was reached, the maximum load was maintained for 1 h to attain homogeneity of the sample.

Table 1 Physical properties of the bentonite and sand used

Bentonite	Sand
Specific gravity $G_s = 2.71$ Liquid limit (%) $w_{I} = 253.5$	Specific gravity $G_s = 2.65$ Uniformity coefficient $U_c = 1.7$
Plastic limit (%) $w_p = 28.6$ Plasticity index $I_p = 224.9$ Smectite content (%) $C_m = 77.3$	Granularity (mm) $d = 0.5-1.0$
Cation exchange capacity (meq/g), $CEC = 0.517$	

Fig. 1 Schematic diagram of the experimental device for swelling deformation

Next, the sample, together with confining ring, was placed in the experimental device (Fig. 1). This device had a hermetically sealed swelling cell within which water percolated from the inlet into the sample and then flowed out to a filtrate bottle. In the swelling tests, the samples were subjected to various target vertical pressure levels (25, 50, 100, 200, 400, or 800 kPa). Distilled water was passed through the porous samples to achieve hydration. A dial gauge was used to record the swelling strain, defined as the ratio of height increment to initial height of samples. Samples were considered to be fully saturated when the dial gauge reading remained constant for 24 h, and the final swelling strain was at a maximum. Hydration using distilled water lasted for 15–20 days and ~200 mL of water was consumed. After full saturation, the pH values of sample leachates were measured $(\sim 7.68.7)$ which revealed that only a small amount of the NaOH was left in the samples.

Mineral Composition and Microstructure Tests

After allowing the samples to swell in distilled water, the pure bentonite samples were dried at 105°C for 24 h and ground into powder using a rubber pestle. The microstructure of the NaOH-treated bentonite-sand samples was examined using a Hitachi H-9000NAR transmission electron microscope (TEM) (Hitachi, Tokyo, Japan). Before recording TEM observations, the bentonite powder was applied to and spread uniformly over an electron microscope slide. Care was taken to ensure that each slice of sample was between 0.1 and 0.2 mm thick.

The untreated and NaOH-treated pure bentonite samples were tested after different reaction times using a Bruker D8 Advance X-ray powder diffractometer (XRD) (Bruker, Billerica, Massachusetts, USA) with CuKα radiation, and the mineral compositions were analyzed using X'Pert HighScore software.

RESULTS AND DISCUSSION

Mineral Composition

Based on the error analysis of XRD, the greater the mineral content, the lesser the relative error. The variations in mineral compositions of pure bentonite after various dissolution processes are listed in Table 2, and the accuracy of X'Pert HighScore software is also mentioned. After each reaction time, the mineral contents of five samples tested were averaged. Because the swelling of the bentonite tested was observed to depend largely on the smectite content rather than on other minerals, only the smectite content was taken into consideration. The smectite content exceeded 60% in the case of all pure bentonite specimens during the dissolution process, and it decreased with the increase in reaction time. The dissolution rate of smectite was faster early in the reaction process, but slowed as the

Table 2 Mineral compositions (wt.%) of pure bentonite samples during dissolution over various reaction times

Reaction time (months)	Smectite	Quartz	Feldspar
$\mathbf{0}$	77.3	16.3	6.1
6	68.4	19.5	11.7
12	66.1	20.4	13.2
24	64.2	20.9	14.5

Mineral content $\geq 40\%$, relative error $\leq 10\%$; Mineral content = 20– 40%, relative error \leq 20%; Mineral content = 5–20%, relative error ≤30%

Fig. 2. Maximum swelling strain vs. vertical stress on a untreated bentonite-sand mixtures and mixtures dissolved by alkaline solution for b 6, c 12, and **d** 24 months with $\rho_d = 1.7$ g/cm³

concentration of OH– in porewater decreased gradually under the testing conditions.

Results of Swelling Tests

The maximum swelling strain was observed to be influenced by the vertical pressure, the reaction time, and the bentonite content (Fig. 2). The maximum strain decreased with an increase in vertical overburden pressure, but increased with increases in the bentonite content of the bentonite-sand mixtures. When other prevailing conditions were kept constant, the untreated bentonite and the bentonite-sand mixtures exhibited greater ε_{max} values than the samples treated with alkaline NaOH solution. As the reaction time increased, the bentonite content dropped continually in the mixtures due to smectite dissolution that ultimately led to a decrease in the maximum swelling strain.

e_c - σ Relationship for Mixtures

The swelling of bentonite-sand mixtures can also be de-scribed by the void ratio parameter (Studds et al. [1998;](#page-7-0) Sun et al. [2009](#page-7-0)). The void ratio of a clay mixture, ec, at full saturation is expressed as:

$$
e_{\rm c} = \frac{V_{\rm w}}{V_{\rm c}} = e \frac{d_{\rm c}}{d_{\rm M} \alpha} \tag{1}
$$

where V_w and V_c are the volume of water and bentonite, respectively; e is the total void ratio of the bentonite-sand mixture, d_c is the relative density of bentonite, and d_M is the average relative density of the bentonite-sand mixture.

The void ratio of pure bentonite decreased with increasing reaction time (Fig. 3). In addition, for samples dissolved in NaOH solution for the same reaction time, the plots of void ratio vs. pressure can be expressed by a uniform fractal e_c -σ relationship expressed as (Xu et al. [2003\)](#page-7-0):

$$
e_{\rm c} = \lambda \sigma^{D_{\rm s}-3} \tag{2}
$$

where λ is the swelling coefficient of bentonite, σ is the vertical pressure, and D_s is the surface fractal dimension.

According to the e_c - σ relationships for the untreated pure bentonite and alkaline-dissolved samples (Table [3\)](#page-4-0), the swelling coefficient, λ , was found to decrease gradually with an increase in reaction time, such that the λ value was smallest for samples treated with alkaline solution for 24 months. By comparing Tables [2](#page-2-0) and [3](#page-4-0), the λ value decreased with the decrease in smectite content.

Fig. 3. Void ratio at saturation vs. vertical pressure on pure bentonite samples

Sample	e_{c} - σ relation	R^2		
Untreated bentonite	$e = 7.954 \sigma^{-0.334}$	0.992	7.954	2.666
Dissolved for 6 months	$e = 6.396 \sigma^{-0.306}$	0.994	6.196	2.694
Dissolved for 12 months	$e = 5.181 \sigma^{-0.293}$	0.993	5.181	2.707
Dissolved for 24 months	$e = 4.215 \sigma^{-0.279}$	0.995	4.215	2.721

Table 3 e_c - σ relation for pure bentonite samples dissolved by alkaline solution for various reaction times

The D_s values increased slightly with increases in reaction time, as shown in Table 3 . D_s was introduced to represent the complexity of the microstructure based on the similarity of surfaces at different scales; $D_s \approx 2-3$, and rougher surfaces have larger D_s values (Xiang et al. [2019b](#page-7-0)). For bentonite, D_s was dependent mainly on the morphologies of the minerals, the smectite in particular. One layer of smectite is almost 0.96 nm thick and 100–1000 nm wide. Examination of the surface morphology of smectite must, therefore, be undertaken at the nanoscale. The resolution of TEM approaches the sub-Ångstrom level because this technique uses an electron beam of much shorter wavelength than an optical light source. Furthermore, TEM offers a unique opportunity to visualize directly the morphology of the nanoparticles. Transmission electron microscopy images at a resolution of 20 nm were selected for observing the surface morphology of smectite dissolved by the

alkaline solution (Fig. 4). Dissolution by alkaline solution caused dissolution traces on the surface of smectite which increased the irregularity and the fractality of the bentonite surface.

The estimation values from the e_c -σ relation (listed in Table 3) also fitted well with the experimental data for the bentonite-sand mixtures with large bentonite contents (α = 70%) after the respective reaction times (Fig. [5\)](#page-5-0). This type of mixture possessed the same e_c value as that of the pure bentonite upon dissolution by the alkaline solution over the same time and under the same vertical pressure.

Threshold of Sand-Particle Contact

The void ratio of clay in the mixtures with intermediate bentonite contents ($\alpha = 50\%$) also decreased with increased reaction duration (Fig. [6\)](#page-5-0). For the untreated samples and the

Fig. 4. TEM images of a untreated bentonite and samples reacted with NaOH solution for b 6, and c 24 months

Fig. 5. Comparison between estimates and experimental data for the void ratio of clay in mixtures with 70% initial bentonite content

samples dissolved by alkaline solution over a period of 6 months, the experimental data can be arranged with the e_c -σ curves as expressed for the pure bentonite. For samples after treatment with alkaline solution for 12 and 24 months, the tested plots deviated from the e_c - σ curves once the vertical pressure exceeded the threshold.

The void ratios of clays in the low-bentonite ($\alpha = 45\%$) mixtures began to deviate from the predicted values when the pressure exceeded a threshold (Fig. 7).

The pressure threshold also decreased as the reaction time increased. According to the results, the pressure threshold, above which the deviation began, depended on the initial bentonite content in the mixtures as well as the reaction time in the alkaline solution. Generally, the pressure threshold decreased with the decrease in the bentonite content in the case of deviation occurring. A threshold of bentonite content also emerged, below which the deviation of void ratio can occur, but above which deviation did not occur. In the case of mixtures with large bentonite contents, the bentonite was able to swell against the vertical pressure and separate the sand particles. The pressure was, therefore, all borne by the swelling clay particles, and the volume changes of the mixtures followed the e_c -σ curve. On the other hand, in the case of mixtures with

small bentonite contents, under greater pressure, the sand particles were still in contact with each other and formed a skeleton of sand particles. In that case, not all of the pressure was borne by the bentonite alone and a larger e_c than in the case of pure bentonite developed. For the tested condition, the alkaline solution had little effect on the sand content of bentonite-sand mixtures but had a significant effect on the bentonite content. As the clay content decreased with the increase in reaction time, the deviation was more likely to occur.

The skeleton void ratio e_s can be expressed as (Sun et al. [2015](#page-7-0)):

$$
e_{\rm s} = \frac{V_{\rm w} + V_{\rm c}}{V_{\rm sd}}\tag{3}
$$

where V_c is the volume of bentonite in the mixture, V_{sd} is the volume of sand particles, and $V_w + V_c$ represents the volume excluding sand particles in the mixture. According to Eq. [1](#page-3-0), $V_w = e_c V_c$ is satisfied at full saturation, and thus the relationship between e_c and e_s can be expressed as:

$$
e_{\rm c} = Ae_{\rm s} - 1\tag{4}
$$

with

$$
A = \frac{V_{sd}}{V_c} = \frac{1 - \alpha}{\alpha} \frac{d_c}{d_{sd}}
$$
 (5)

where d_{sd} is the relative density of sand particles. In this study, $d_c = 2.71$ and $d_{sd} = 2.65$.

The sand skeleton can only be formed when $V_{sd} > (V_w + V_c)$ (Xiang et al. [2019a](#page-7-0)); the measured data are bound to follow the e_c -σ curve at $e_s \geq 1.0$, whereas the discrepancy appears when e_s < 1. If, for instance, the deviation begins from the unique e_c -σ curve, the e_s is expected to approach 1.0, where the fractal e_c - σ relation is simultaneously satisfied as depicted in Eq. 6. That is:

$$
1.0A-1 = e_c = \lambda \sigma^{D_s-3}
$$
 (6)

Fig. 6. e_c - σ relationship for mixtures with 50% initial bentonite content

Fig. 7. e_c - σ relationship for mixtures with 45% initial bentonite content

The bentonite content at which the sand skeleton starts breaking apart can be calculated on the assumption that it happens as soon as the bentonite starts to take up water ($e_c = 0$); according to Eq. [6,](#page-5-0) the limit of bentonite content in the tested bentonite was estimated to be ~50%. Therefore, at this intermediate bentonite dosage of 50%, deviation is not likely to occur. Due to the dissolution by the alkaline solution, however, the bentonite content decreased continually, and, therefore, the deviation varied with reaction time, as illustrated in Fig. [6](#page-5-0).

Rearranging Eq. [6](#page-5-0) gives:

$$
\log \sigma = \frac{\log(A-1) - \log \lambda}{D_s - 3} \tag{7}
$$

The threshold of vertical pressure, σ_s , can be calculated from Eq. 7. For the mixture with 45% initial bentonite content dissolved in alkaline solution over 0, 6, 12, and 24 months, the estimated thresholds of pressure, σ_s , are 92.2, 77.7, 61.2, and 43.2 kPa, respectively.

Many experimental data indicated that the e_c - σ relations for bentonite-sand mixtures were still a straight line in the double logarithmic scale after the sand skeleton forms (Sun et al. [2015;](#page-7-0) Xiang et al. [2019a\)](#page-7-0), which can also be expressed as:

$$
\log e_c = \log a - b \log \sigma \tag{8}
$$

with

$$
b = \frac{\log e_c - \log e_{cm}}{\log \sigma - \log \sigma_s}
$$
 (9)

where $e_{\rm cm}$ is the clay void ratio of mixtures under the starting deviation stress, σ_s . Also shown in Fig. [7](#page-5-0) is the e_c - σ relation for mixtures with 45% initial bentonite content, while the solid lines and dotted lines are the estimated values calculated using Eqs. [4](#page-5-0) and 8, respectively, which agreed well with the experimental data.

CONCLUSIONS

The bentonite contents of bentonite-sand mixtures decreased gradually with increased reaction times in alkaline NaOH solution, which ultimately decreased the swelling deformation. The void ratio of pure bentonite which reacted with NaOH solution for the same reaction time can be expressed as $e = \lambda p_s^{D_s - 3}$. The swelling coefficient, λ , decreased with increase in reaction time due to bentonite dissolution. The increase in the fractal dimension, D_s , was caused by increased dissolution traces on bentonite particle surfaces, as observed by TEM.

The volumetric change in bentonite-sand mixtures with large bentonite contents ($\alpha = 70\%$) followed the fractal e_c -σ curves, as the pure bentonite reacted with the alkaline solution for the same amount of time. For mixtures with $\alpha = 45\%$ under greater pressure, sand-skeleton formation occurred and this sustained part of the load. Consequently, the experimental data deviated from the e_c - σ curve relationship and, thus, resulted in a larger e_c value than that for pure bentonite. The bentonite

limit of the tested bentonite was estimated to be ~50%. No deviation in the void ratio was found in untreated mixtures containing 50% bentonite. However, with increased alkaline solution reaction times, the bentonite content decreased continuously. The void ratio deviation was found in bentonitesand mixtures that reacted with alkaline solution over periods of 12 and 24 months. In addition, the vertical pressure threshold values were also estimated using the e_c -σ relation. Under the test conditions, the vertical pressure threshold decreased with decreasing initial bentonite content, as well as decreased with increasing reaction time.

These conclusions were based on limited experimental conditions because the bentonite-sand mixtures and alkaline solution were tested in a closed system at room temperature. Hence, further studies are required to apply the aforementioned experimental results to other conditions.

ACKNOWLEDGMENTS

The National Natural Science Foundation of China (Grant No. 41702311) and China Postdoctoral Science Foundation (2019M660096) are acknowledged for their financial support.

FUNDING

Funding is as stated in the acknowledgments.

Compliance with ethical standards

Conflicts of interest

The authors wish to confirm that no known conflicts of interest are associated with this publication and no significant financial support of this work had any influence or bias on the conclusions.

REFERENCES

- Alonso, E.E., Vaunat, J., & Gens, A. (1999) Modeling the mechanical behavior of expansive clays. Engineering Geology, 54, 173–183.
- Ashmawy, A.K., El-Hajji, D., Sotelo, N., & Muhammad, N. (2002) Hydraulic performance of untreated and polymer-treated bentonite in inorganic landfill leachates. Clays and Clay Minerals, 50, 546– 552.
- Bolt, G.H. (1956) Physico-chemical analysis of the compressibility of pure clays. Geotechnique, 6, 86–93.
- Charpentiera, D., Devineau, K., Mosser-Ruck, R., Cathelineau, M., & Villieras, F. (2006) Bentonite-iron interactions under alkaline condition: an experimental approach. Applied Clay Science, 32, 1–13.
- Chen, Y.G., Sun, Z., Cui, Y.J., Ye, W. M., & Liu, Q.H. (2019) Effect of cement solutions on the swelling pressure of compacted GMZ bentonite at different temperatures. Construction and Building Materials, 229, 116872.
- Cuadros, J. & Linares, J. (1996) Experimental kinetic study of the smectite-to-illite transformation. Geochimica et Cosmochimica Acta, 60, 439–453.
- Cuevas, J., De la Villa, R. V., Ramírez, S., Sánchez, L., Fernández, R., & Leguey, S. (2006) The alkaline reaction of FEBEX bentonite: a contribution to the study of the performance of bentonite/concrete engineered barrier systems. Journal of Iberian Geology, 32, 151-174.
- Cuisinier, O., Masrouri, F., Pelletier, M., Villieras, F., & Mosser-Ruck, R. (2008) Microstructure of a compacted soil submitted to an alkaline plume. Applied Clay Science, 40, 159–170.
- Du, Y.J., Fan, R.D., Liu, S.Y., Reddy, K.R., & Jin, F. (2015) Workability, compressibility and hydraulic conductivity of zeolite-

amended clayey soil/calcium-bentonite backfills for slurry-trench cutoff walls. Engineering Geology, 195, 258–268.

- Fernández, R., Cuevas, J., Sánchez, L., de la Villa, R.V., & Leguey, S. (2006) Reactivity of the cement–bentonite interface with alkaline solutions using transport cells. Applied Geochemistry, 21, 977–992.
- Gates, W.P. & Bouazza, A. (2010) Bentonite transformations in strongly alkaline solutions. Geotextiles & Geomembranes, 28, 219–225.
- Gaucher, E.C. & Blanc, P. (2006) Cement/clay interactions a review: experiments, natural analogues, and modeling. Waste Manage, 26, 776–788.
- Hayakawa, T., Minase, M., Fujita, K.I., & Ogawa, M. (2016) Modified method for bentonite purification and characterization; A case study using bentonite from Tsunagi Mine, Niigata, Japan. Clays and Clay Minerals, 64, 275–282.
- Heikola, T., Kumpulainen, S., Vuorinen, U., Kiviranta, L., & Korkeakoski, P. (2013) Influence of alkaline (pH 8.3 – 12.0) and saline solutions on chemical, mineralogical and physical properties of two different bentonites. Clay Minerals, 48, 309–329.
- Herbert, H., Kasbohm, J., Sprenger, H., Fernández, A.M., & Reichelt, C. (2008) Swelling pressures of MX-80 bentonite in solutions of different ionic strength. Physics and Chemistry of the Earth, 33, S327–S342.
- Hoeks, J., Glas, H., Hofkamp, J., & Ryhiner, A. (1987) Bentonite liners for isolation of waste disposal sites. Waste Management & Research, 5, 93–105.
- Karnland, O., Olsson, S., Nilsson, U., & Sellin, P. (2007) Experimentally determined swelling pressures and geochemical interactions of compacted Wyoming bentonite with highly alkaline solutions. Physics and Chemistry of the Earth, 32, 275–286.
- Komine, H. & Ogata, N. (1996) Prediction for swelling characteristics of compacted bentonite. Canadian Geotechnical Journal, 33, 11– 22
- Mollins, L.H., Stewart, D.I., & Cousens, T.W. (1996) Predicting the properties of bentonite-sand mixtures. Clay Minerals, 31, 243–252.
- Nakayama, S., Sakamoto, Y., Yamaguchi, T., Akai, M., Tanaka, T., Sato, T., & Iida, Y. (2004) Dissolution of montmorillonite in compacted bentonite by highly alkaline aqueous solutions and diffusivity of hydroxide ions. Applied Clay Science, 27, 53–65.
- Ramı́ rez, S., Cuevas, J., Vigil, R., & Leguey, S. (2002) Hydrothermal alteration of "La Serrata" bentonite (Almeria, Spain) by alkaline solutions. Applied Clay Science, 21, 257–269.
- Roscoe, K.H. & Burland, J.B. (1968) On the generalized stress–strain behaviour of the 'wet' clay. Pp. 535–609 in: Engineering Plasticity (J. Heyman and F.A. Leckie, editors). Cambridge University Press, Cambridge, UK.
- Sánchez, L., Cuevas, J., Ramírez, S., De León, D.R., Fernández, R., Villa, R.V.D., & Leguey, S. (2006) Reaction kinetics of FEBEX bentonite in hyperalkaline conditions resembling the cement– bentonite interface. Applied Clay Science, 33, 125–141.
- Savage, D., Walker, C., Arthur, R., Rochelle, C., Oda, C., & Takase, H. (2007) Alteration of bentonite by hyperalkaline fluids: A review of

the role of secondary minerals. Physics and Chemistry of the Earth, Parts A/B/C, 32, 287–297.

- Schanz, T. & Tripathy, S. (2009) Swelling pressures of a divalent-rich bentonite: diffuse double layer theory revisited. Water Resources Research, 45, W00C12.
- Studds, P.G., Stewart, D.I., & Cousens, T.W. (1998) The effects of salt solutions on the properties of bentonite-sand mixtures. Clay Minerals, 33, 651–660.
- Sun, D.A., Cui, H., & Sun, W. (2009) Swelling of compacted sand– bentonite mixtures. Applied Clay Science, 43, 485-492.
- Sun, W.J., Wei, Z.F., Sun, D.A., Liu, S.Q., Fatahi, B., & Wang, X.Q. (2015) Evaluation of the swelling characteristics of bentonite–sand mixtures. Engineering Geology, 199, 1–11.
- Tripathy, S., Sridharan, A., & Schanz, T. (2004) Swelling pressures of compacted bentonites from diffuse double layer theory. Canadian Geotechnical Journal, 41, 437–450.
- Villar, M.V. (2007) Water retention of two natural compacted bentonites. Clays and Clay Minerals, 55, 311–322.
- Wang, Q., Tang, A.M., Cui, Y.J., Delage, P., & Gatmiri, B. (2012). Experimental study on the swelling behavior of bentonite/claystone mixture. Engineering Geology, 124, 59–66.
- Xiang, G.S., Yu, F., Xu, Y.F., Fang, Y., & Xie, S.H. (2019a) Prediction for swelling deformation of fractal-textured bentonite and its sandmixtures in salt solution. Clay Minerals, 54, 161–167.
- Xiang, G.S., Ye, W.W., Yu, F., Wang, Y., & Fang, Y. (2019b) Surface fractal dimension of bentonite affected by long-term corrosion in alkaline solution. Applied Clay Science, 175, 94–101.
- Xu, Y.F., Matsuoka, H., & Sun, D.A. (2003) Swelling characteristics of fractal-textured bentonite and its mixtures. Applied Clay Science, 22, 197–209.
- Xu, Y.F., Xiang, G.S., Jiang, H., Chen, T., & Chu, F.F. (2014) Role of osmotic suction in volume change of clays in salt solution. Applied Clay Science, 101, 354–361.
- Yamaguchi, T., Sakamoto, Y., Akai, M., Takazawa, M., Iida, Y., Tanaka, T., & Nakayama, S. (2007) Experimental and modeling study on long-term alteration of compacted bentonite with alkaline groundwater. Physics and Chemistry of the Earth, Parts A/B/C, 32, 298–310.
- Ye, W.M., He, Y., Chen, Y.G., Chen, B., & Cui, Y.J. (2016) Thermochemical effects on the smectite alteration of GMZ bentonite for deep geological repository. Environmental Earth Sciences, 75, 906.
- Yong, R.N. (1999) Soil suction and soil-water potentials in swelling clays in engineered clay barriers. Engineering Geology, 54, 3–13.

(Received 15 January 2020; revised 24 June 2020; AE: William F. Jaynes)